Static Design Criteria Failure Theories

Overview

- In case of uniaxial loading, there is only one value of stress, and one value of strength.
- Stress: $\sigma = F/A$ Strength: S_v
- What other types of uniaxial loading do we have?
- It is easy to evaluate safety. n=S_y/ σ
- What do we do in case of multi-axial loading to evaluate safety?

Overview

- We use Failure Theories
- The Failure theories that we are going to consider are as follows:
	- Maximum Normal Stress Theory
	- Maximum Shear Stress Theory
	- Distortion Energy Theory
	- Coulomb Mohr Theory
	- Modified Mohr Theory

• According to MNST failure occurs whenever one of the three principal stresses equals the strength.

- Let $\sigma_1 > \sigma_2 > \sigma_3$
- According to MNST failure occurs whenever
- $\sigma_1 = S_t$ or $\sigma_3 = -S_c$
- S_t and S_c are tensile and compressive strengths, respectively.
- MNST is usually applied to brittle failure.
- In this case S_t and S_c are ultimate strengths.

- In case MNST is applied to a ductile failure S_t and S_c are yield strengths.
- Safe combinations of σ_1 , σ_2 , σ_3 can be represented geometrically in the so called "stress space".
- In the stress space, the principal stresses $(\sigma_1, \sigma_2, \sigma_3)$ at a given point in the material represent the coordinates of a point.

Maximum Normal Stress Theory σ_1 σ_2 σ_3 **A** F_1 $F₂$ T_1 σ_1 σ_2 σ_3 \overline{O}

F3

F4

A is a point in a machine element

• According to MNST safe combinations of σ_1 , σ_2 , σ_3 are inside the cube shown.

- S_t and S_c need not be equal
- In case of biaxial loading (where one of the principal stresses is always zero), safe combinations of principal stresses ($\sigma_{\mathsf{A}}, \sigma_{\mathsf{B}}$) are inside the square shown in 2D stress space.
- When do we have one of the principal stresses is always zero?

- MNST is very easy to use, unfortunately it has a serious shortcoming for ductile materials.
- For ductile materials, yield strengths in tension and compression are usually the same. $S_{yt} = S_{yc} = S_y$.
- Prediction of MNST may become unconservative if the largest and the smallest principal stresses have opposite signs.

- To see this consider the case of pure torsion
- Take a section across point B which is on the surface.

 τ_{xy} is the only nonzero stress.

For a point B on the surface, 3D Mohr circle is as follows:

• As the applied torque T increases, pricipal stresses describe a straight line in stress space whose equation is:

 $\sigma_1 = - \sigma_3$

• This line is called the "loading line".

$$
\text{Let } -S_c = S_t = S_y
$$

- So, MNST predicts that failure (onset of yielding) will occur when,
- $\sigma_1 = S_y$ where S_y is yield strength in uniaxial tension,
- But at the same time this implies, at failure
- $\tau_{xy} = S_y$, since $\tau_{xy} = \sigma_1$!
- This prediction does not agree with experiments.

- Yield strength under pure shear, S_{sv} can be measured with torsion experiments.
- These experiments show that for ductile materials, S_{sy} is about 60% of S_{y} whereas MNST predicted that they were equal.
- For this reason MNST is not recommended for ductile materials.

- On the other hand for brittle materials, it is observed from experiments that Ultimate Tensile Strength is roughly equal to Ultimate Shear Strength.
- $S_{\rm ut} = S_{\rm su}$
- So MNST can be used for brittle materials

- This theory is also known as Tresca Yield Condition.
- It is proposed to predict yielding, so it is applicable to ductile materials.
- According to MSST, yielding begins whenever the max. shear stress in any mechanical element becomes equal to the max. shear stress in a tension test specimen of the same material when that specimen begins to yield.

• For uniaxial tension test, from the 3D Mohr circle, one can observe that the maximum shear stress is;

$$
\bullet \quad \tau_{max} = \sigma_1/2 = S_y/2
$$

- Therefore this theory implies that yield strength in shear, S_{sv} is half of the tensile yield strength.
- $S_{sy} = S_y/2$
- For a general 3D stress state, let the principal stresses be;

 $\sigma_1 > \sigma_2 > \sigma_3$

- According to MSST, yielding will occur as soon as $\tau_{\text{max}} \geq$ Ssy=Sy/2
- In other words, $\sigma_1 \sigma_3 \geq S_v$

• In general maximum shear stresses in 12, 23 and 13 planes are given by

•
$$
\tau_{1/2} = (\sigma_1 - \sigma_2)/2
$$
,

- $\tau_{2/3} = (\sigma_2 \sigma_3)/2$,
- $\tau_{1/3} = (\sigma_1 \sigma_3)/2$
- As soon as one of these shear stresses becomes equal to $S_{sy} = S_y/2$, yielding begins.
- Sign of the shear stress indicates their direction.

- Then the following six equations define 6 planes. The volume inside these planes is the safe region in 3D stress space.
- $\sigma_1 \sigma_2 = \pm S_v$
- $\sigma_2 \sigma_3 = \pm S_v$
- $\sigma_1 \sigma_3 = \pm S_v$
- Planes form an obligue regular hexagonal prism with hydrostatic line as its axis.

 σ

Axis

σ-

 \overline{S}

 S_{ν}

 S_{ν}

- Hydrostatic line
- $\sigma_1 = \sigma_2 = \sigma_3$

• Note that we can write stresses as follows:

$$
\sigma_1 = \sigma_1' + \sigma_1''
$$
\n
$$
\sigma_2 = \sigma_2' + \sigma_2''
$$
\nwhere\n
$$
\sigma_2 = (2\sigma_1 - \sigma_2 - \sigma_3)/3
$$
\n
$$
\sigma_3 = \sigma_3' + \sigma_3''
$$
\n
$$
\sigma_3 = (2\sigma_3 - \sigma_1 - \sigma_2)/3
$$
\nand\n
$$
\sigma_1' = \sigma_2'' = \sigma_3'' = (\sigma_1 + \sigma_2 + \sigma_3)/3
$$

- Components with double prime are called hydrostatic components
- Components with single prime are called deviatoric components
- We can easily observe that, at yielding,

$$
\sigma_1 - \sigma_2 = \sigma_1' - \sigma_2' = \pm S_y
$$
, $\sigma_2 - \sigma_3 = \sigma_2' - \sigma_3' = \pm S_y$
 $\sigma_1 - \sigma_3 = \sigma_1' - \sigma_3' = \pm S_y$

• This means that according to this theory, no matter how large it may be, hydrostatic stress has no effect on yielding.

 σ

 σ

 $\overline{\sigma}$

- For a biaxial case let $\sigma_3=0$, and σ_A and σ_B be the non-zero principal stresses.
- In this case, the safe region in the 2D stress state is as shown below.
- Note that this the intersection of the obligue hexagonal prism with the $\sigma_3=0$ plane.

- Shaded area represents the safe region. (i.e. safe combinations of σ_A and σ_B)
- These σ_A and σ_B satisfy all of the six inequalities.

$$
\sigma_A\!-\!\sigma_B\!\leq S_y \qquad \quad \sigma_B\!\leq S_y \qquad \quad \sigma_A\!\leq S_y
$$

 $\sigma_A-\sigma_B \geq -S_v$ $\sigma_B \geq -S_v$ $\sigma_A \geq -S_v$

- Note that in the 1st and the 3rd quadrant MNST and MSST make the same prediction.
- 2nd and 4th zones where principal stresses have opposite sign differ.
- In design problems an effective (equivalent) stress can be defined as follows:

- Let $\sigma_1 > \sigma_2 > \sigma_3$
- \bullet σ' $= \sigma_1 - \sigma_3$
- According to MSST, yielding will occur as soon as $\sigma' \geq S_y$
- Then factor of safety, $n=S_{\rm v}/\sigma'$

- Strain energy stored in a unit volume of element due to combined loading can be split up into two parts.
- Strain energy associated with the volume change
- Strain energy associated with distortion

- Let $\sigma_1 > \sigma_2 > \sigma_3$
- $\sigma_{\text{ave}} = (\sigma_1 + \sigma_2 + \sigma_3)/3$

• Recall that Strain Energy per unit volume is given in terms of principal stresses as follows:

$$
u = \frac{1}{2E} \Big(\sigma_1^2 + \sigma_2^2 + \sigma_3^2 - 2\nu \Big(\sigma_1 \sigma_2 + \sigma_2 \sigma_3 + \sigma_1 \sigma_3 \Big) \Big)
$$

• If we substitute $\sigma_1=\sigma_2=\sigma_3=\sigma_{ave}$ in the equation above, we get strain energy per unit volume associated with volume change.

Distortion Energy Theory $\frac{\sigma_{ave}}{\sigma_{E}}(1-2\nu)$ $1 - 2$ 2 $3\sigma^{-2}$ $=$ $-$ *E* $u_{\mu} = \frac{1 - a\nu e}{a}$ *v*

• than strain energy per unit volume associated with distortion (DE) is given as follows:

$$
u_{d} = u - u_{v} = \frac{1 + v}{3E} \left(\frac{(\sigma_{1} - \sigma_{2})^{2} + (\sigma_{2} - \sigma_{3})^{2} + (\sigma_{3} - \sigma_{1})^{2}}{2} \right)
$$

• Note that DE is zero for hydrostatic stress state $(\sigma_1 = \sigma_2 = \sigma_3)$.

- DET predicts that yielding will occur whenever the DE equals the distortion energy in the same volume, when it is uniaxially stressed to the yield strength.
- For simple tension test, let $\sigma_1=\sigma'$, $\sigma_2=\sigma_3=0$ in the expression for DE:

$$
(u_d)_{\text{uni.}} = \frac{1+\nu}{3E} \sigma^2
$$

• Equating the DE for uniaxial case to general expression of DE and solving for σ' .

$$
\sigma' = \sqrt{\frac{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_1 - \sigma_3)^2}{2}}
$$

- Yielding will occur when $\sigma' = S_y$.
- \bullet σ' which is the effective stress, representing the entire stress state is called Von Mises stress.

- For biaxial stress state, let σ_A and σ_B be the two non zero principal stresses.
- Then Von Mises stress becomes equal to

$$
\bullet \quad \sigma' = (\sigma_A^2 - \sigma_A \sigma_B + \sigma_B^2)^{1/2}
$$

For pure torsion Mohr circle becomes;

$$
\sigma_A = -\sigma_B = \tau_{max} = T.c/J
$$

• Then Von Mises stress for pure torsion becomes equal to

$$
\sigma' = \sqrt{\tau_{\text{max}}^2 - \tau_{\text{max}}(-\tau_{\text{max}}) + (-\tau_{\text{max}})^2}
$$

$$
\sigma' = \sqrt{3}\tau_{\text{max}}
$$

- Then yielding under pure torsion occurs when *y* ı $\sigma' = \sqrt{3}\tau_{\text{max}} = S$
	- This implies yield strength under pure shear, Ssy, according to DET is;

$$
S_{sy} = \frac{S_y}{\sqrt{3}} \approx 0.577 S_y
$$

- The result S_{sy} =0.577S_y agrees very well with experiments.
- The DET is also called
	- The octahedral shear stress theory
	- The Von Mises Hencky theory
- From

$$
\sigma' = \sqrt{\frac{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_1 - \sigma_3)^2}{2}} = S_y
$$

• The equation of the surface enclosing the safe region in stress space is:

$$
(\sigma_1-\sigma_2)^2+(\sigma_2-\sigma_3)^2+(\sigma_1-\sigma_3)^2=2S_y^2
$$

- This equation describes an obligue elliptical cylinder with hydrostatic line as its axis.
- Safe combinations of principal stresses are inside this cylinder.

• Instead of principal stresses, one can calculate Von Mises stress in terms of a given stress state.

$$
\sigma' = \sqrt{\frac{(\sigma_x - \sigma_y)^2 + (\sigma_y - \sigma_z)^2 + (\sigma_x - \sigma_z)^2 + 6(\tau_{xy}^2 + \tau_{yz}^2 + \tau_{xz}^2)}{2}}
$$

• Safety Factor can be calculated as;

$$
n=\frac{S_y}{\sigma'}
$$

• For biaxial case, the safe region is an elliptical are as shown. $\sigma_{\rm B}$ S_{v} σ_{A} S_v Let $-S_c=S_t=S_v$ 45 Loading line for pure torsion $S_{\rm v}$ $-S_v$ $\sigma_A^2 - \sigma_A \sigma_B + \sigma_B^2 = S_y^2$

Comparison of Failure Criteria

Comparison of Failure Criteria

- Note that MNST is actually useful for brittle materials.
- MSST and DET are useful for ductile materials (for predicting onset of yielding).
- MSST is more conservative than DET.
- Agreement of DET with experiments is better.

Comparison of Failure Criteria

• A common type of loading is combined bending and torsion.

- $\mathbb{F}_{\mathbb{A}}$ \mathbb{F}_{xy} In this case principal stresses are given as 2 2 2 W 2 $\sigma_B = \frac{\sigma_x}{2} \pm \sqrt{\left|\frac{\sigma_x}{2}\right|} + \tau_{xy}$ $x + \parallel \mathbf{v}_x$ A_4 , $\sigma_B = \frac{A}{2} \pm \sqrt{|\frac{A}{2}|^2 + \tau^2}$ $\sigma_A, \sigma_B = \frac{\sigma_x}{2} \pm \sqrt{\frac{\sigma_x}{2} + \frac{\sigma_y}{2}}$ \int \setminus \mathbf{I} \setminus $\bigg($ $=\frac{6}{x}$ +
- Effective stresses are given as MSST: DET:

$$
\sigma' = \sigma_A - \sigma_B = 2\sqrt{\left(\frac{\sigma_x}{2}\right)^2 + \tau_{xy}^2}
$$

$$
\sigma' = \sqrt{{\sigma_A}^2 - {\sigma_A}{\sigma_B} + {\sigma_B}^2} = \sqrt{{\sigma_x}^2 + 3{\tau_{xy}}^2}
$$