#### Static Design Criteria Failure Theories

### Overview

- In case of uniaxial loading, there is only one value of stress, and one value of strength.
- Stress : σ=F/A Strength: S<sub>v</sub>
- What other types of uniaxial loading do we have?
- It is easy to evaluate safety.  $n=S_v/\sigma$
- What do we do in case of multi-axial loading to evaluate safety?

# Overview

- We use Failure Theories
- The Failure theories that we are going to consider are as follows:
  - Maximum Normal Stress Theory
  - Maximum Shear Stress Theory
  - Distortion Energy Theory
  - Coulomb Mohr Theory
  - Modified Mohr Theory

 According to MNST failure occurs whenever one of the three principal stresses equals the strength.



- Let  $\sigma_1 > \sigma_2 > \sigma_3$
- According to MNST failure occurs whenever
- $\sigma_1 = S_t$  or  $\sigma_3 = -S_c$
- S<sub>t</sub> and S<sub>c</sub> are tensile and compressive strengths, respectively.
- MNST is usually applied to brittle failure.
- In this case S<sub>t</sub> and S<sub>c</sub> are ultimate strengths.

- In case MNST is applied to a ductile failure  $S_t$  and  $S_c$  are yield strengths.
- Safe combinations of  $\sigma_1$ ,  $\sigma_2$ ,  $\sigma_3$  can be represented geometrically in the so called "stress space".
- In the stress space, the principal stresses  $(\sigma_1, \sigma_2, \sigma_3)$  at a given point in the material represent the coordinates of a point.

#### Maximum Normal Stress Theory F₁ $\sigma_3$ T₁ $\sigma_2$ Ο $\sigma_1$ $F_2$ Α $\sigma_2$ $\sigma_1$ $\sigma_3$ A is a point in a machine element F<sub>3</sub>

 $F_4$ 

• According to MNST safe combinations of  $\sigma_1$  ,  $\sigma_2$  ,  $\sigma_3$  are inside the cube shown.



- S<sub>t</sub> and S<sub>c</sub> need not be equal
- In case of biaxial loading (where one of the principal stresses is always zero), safe combinations of principal stresses ( $\sigma_A, \sigma_B$ ) are inside the square shown in 2D stress space.
- When do we have one of the principal stresses is always zero?



- MNST is very easy to use, unfortunately it has a serious shortcoming for ductile materials.
- For ductile materials, yield strengths in tension and compression are usually the same.  $S_{yt}=S_{yc}=S_{y}$ .
- Prediction of MNST may become unconservative if the largest and the smallest principal stresses have opposite signs.

- To see this consider the case of pure torsion
- Take a section across point B which is on the surface.



 $\tau_{xy}$  is the only nonzero stress.

For a point B on the surface, 3D Mohr circle is as follows:





 As the applied torque T increases, pricipal stresses describe a straight line in stress space whose equation is:

 $\sigma_1 = -\sigma_3$ 

• This line is called the "loading line".

Let 
$$-S_c = S_t = S_v$$



- So, MNST predicts that failure (onset of yielding) will occur when,
- $\sigma_1 = S_y$  where  $S_y$  is yield strength in uniaxial tension,
- But at the same time this implies, at failure
- $\tau_{xy} = S_y$ , since  $\tau_{xy} = \sigma_1$ !
- This prediction does not agree with experiments.

- Yield strength under pure shear, S<sub>sy</sub> can be measured with torsion experiments.
- These experiments show that for ductile materials,  $S_{sy}$  is about 60% of  $S_y$  whereas MNST predicted that they were equal.
- For this reason MNST is not recommended for ductile materials.

- On the other hand for brittle materials, it is observed from experiments that Ultimate Tensile Strength is roughly equal to Ultimate Shear Strength.
- S<sub>ut</sub>=S<sub>su</sub>
- So MNST can be used for brittle materials

- This theory is also known as Tresca Yield Condition.
- It is proposed to predict yielding, so it is applicable to ductile materials.
- According to MSST, yielding begins whenever the max. shear stress in any mechanical element becomes equal to the max. shear stress in a tension test specimen of the same material when that specimen begins to yield.

 For uniaxial tension test, from the 3D Mohr circle, one can observe that the maximum shear stress is;

• 
$$\tau_{max} = \sigma_1 / 2 = S_y / 2$$



- Therefore this theory implies that yield strength in shear, S<sub>sy</sub> is half of the tensile yield strength.
- $S_{sy} = S_y/2$
- For a general 3D stress state, let the principal stresses be;

•  $\sigma_1 > \sigma_2 > \sigma_3$ 



- According to MSST, yielding will occur as soon as  $\tau_{max} \ge Ssy=Sy/2$
- In other words,  $\sigma_1 \sigma_3 \ge S_y$

 In general maximum shear stresses in 12, 23 and 13 planes are given by

• 
$$\tau_{1/2} = (\sigma_1 - \sigma_2)/2$$
,

- $\tau_{2/3} = (\sigma_2 \sigma_3)/2$ ,
- $\tau_{1/3} = (\sigma_1 \sigma_3)/2$
- As soon as one of these shear stresses becomes equal to  $S_{sy}=S_y/2$ , yielding begins.
- Sign of the shear stress indicates their direction.

- Then the following six equations define 6 planes. The volume inside these planes is the safe region in 3D stress space.
- $\sigma_1 \sigma_2 = \pm S_y$
- $\sigma_2 \sigma_3 = \pm S_y$
- $\sigma_1 \sigma_3 = \pm S_y$
- Planes form an obligue regular hexagonal prism with hydrostatic line as its axis.

Axis

S

Sv

- Hydrostatic line
- $\sigma_1 = \sigma_2 = \sigma_3$

Note that we can write stresses as follows:

$$\sigma_{1} = \sigma_{1}' + \sigma_{1}'' \qquad \sigma_{1}' = (2\sigma_{1} - \sigma_{2} - \sigma_{3})/3$$
  

$$\sigma_{2} = \sigma_{2}' + \sigma_{2}'' \qquad \text{where} \qquad \sigma_{2}' = (2\sigma_{2} - \sigma_{1} - \sigma_{3})/3$$
  

$$\sigma_{3} = \sigma_{3}' + \sigma_{3}'' \qquad \sigma_{3}' = (2\sigma_{3} - \sigma_{1} - \sigma_{2})/3$$
  
and  

$$\sigma_{1}'' = \sigma_{2}'' = \sigma_{3}'' = (\sigma_{1} + \sigma_{2} + \sigma_{3})/3$$

- Components with double prime are called hydrostatic components
- Components with single prime are called deviatoric components
- We can easily observe that, at yielding,

$$\sigma_1 - \sigma_2 = \sigma_1' - \sigma_2' = \pm \mathbf{S}_y \quad \sigma_2 - \sigma_3 = \sigma_2' - \sigma_3' = \pm \mathbf{S}_y$$
$$\sigma_1 - \sigma_3 = \sigma_1' - \sigma_3' = \pm \mathbf{S}_y$$

 This means that according to this theory, no matter how large it may be, hydrostatic stress has no effect on yielding.

σ

σ



- For a biaxial case let  $\sigma_3=0$ , and  $\sigma_A$  and  $\sigma_B$  be the non-zero principal stresses.
- In this case, the safe region in the 2D stress state is as shown below.
- Note that this the intersection of the obligue hexagonal prism with the  $\sigma_3=0$  plane.



- Shaded area represents the safe region. (i.e. safe combinations of  $\sigma_{\rm A}$  and  $\sigma_{\rm B})$
- These  $\sigma_{\text{A}}$  and  $\sigma_{\text{B}}$  satisfy all of the six inequalities.

$$\sigma_{A} - \sigma_{B} \leq S_{y} \qquad \sigma_{B} \leq S_{y} \qquad \sigma_{A} \leq S_{y}$$

 $\sigma_{\!A}\!\!-\!\!\sigma_{\!B}\!\geq\!-\!S_{\!y} \qquad \sigma_{\!B}\!\geq\!-\!S_{\!y} \qquad \sigma_{\!A}\!\geq\!-\!S_{\!y}$ 

- Note that in the 1st and the 3rd quadrant MNST and MSST make the same prediction.
- 2nd and 4th zones where principal stresses have opposite sign differ.
- In design problems an effective (equivalent) stress can be defined as follows:

- Let  $\sigma_1 > \sigma_2 > \sigma_3$
- $\sigma' = \sigma_1 \sigma_3$
- According to MSST, yielding will occur as soon as  $\sigma' \ge S_v$
- Then factor of safety,  $n=S_y/\sigma'$

- Strain energy stored in a unit volume of element due to combined loading can be split up into two parts.
- Strain energy associated with the volume change
- Strain energy associated with distortion



- Let  $\sigma_1 > \sigma_2 > \sigma_3$
- $\sigma_{ave} = (\sigma_1 + \sigma_2 + \sigma_3)/3$

 Recall that Strain Energy per unit volume is given in terms of principal stresses as follows:

$$u = \frac{1}{2E} \left( \sigma_1^2 + \sigma_2^2 + \sigma_3^2 - 2\nu \left( \sigma_1 \sigma_2 + \sigma_2 \sigma_3 + \sigma_1 \sigma_3 \right) \right)$$

• If we substitute  $\sigma_1 = \sigma_2 = \sigma_3 = \sigma_{ave}$  in the equation above, we get strain energy per unit volume associated with volume change.

# Distortion Energy Theory $u_{v} = \frac{3\sigma_{ave}^{2}}{2E}(1-2v)$

 than strain energy per unit volume associated with distortion (DE) is given as follows:

$$u_{d} = u - u_{v} = \frac{1 + v}{3E} \left( \frac{(\sigma_{1} - \sigma_{2})^{2} + (\sigma_{2} - \sigma_{3})^{2} + (\sigma_{3} - \sigma_{1})^{2}}{2} \right)$$

• Note that DE is zero for hydrostatic stress state ( $\sigma_1 = \sigma_2 = \sigma_3$ ).

- DET predicts that yielding will occur whenever the DE equals the distortion energy in the same volume, when it is uniaxially stressed to the yield strength.
- For simple tension test, let  $\sigma_1 = \sigma'$ ,  $\sigma_2 = \sigma_3 = 0$  in the expression for DE:

$$\left(u_d\right)_{uni.} = \frac{1+\nu}{3E}\sigma'^2$$

 Equating the DE for uniaxial case to general expression of DE and solving for σ',

$$\sigma' = \sqrt{\frac{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_1 - \sigma_3)^2}{2}}$$

- Yielding will occur when  $\sigma' = S_v$ .
- σ' which is the effective stress, representing the entire stress state is called Von Mises stress.

- For biaxial stress state, let  $\sigma_A$  and  $\sigma_B$  be the two non zero principal stresses.
- Then Von Mises stress becomes equal to

• 
$$\sigma' = (\sigma_A^2 - \sigma_A \sigma_B + \sigma_B^2)^{1/2}$$

For pure torsion Mohr circle becomes;

$$\sigma_{A} = -\sigma_{B} = \tau_{max} = T.c/J$$



 Then Von Mises stress for pure torsion becomes equal to

$$\sigma' = \sqrt{\tau_{\text{max}}^2 - \tau_{\text{max}} (-\tau_{\text{max}}) + (-\tau_{\text{max}})^2}$$
$$\sigma' = \sqrt{3}\tau_{\text{max}}$$

- Then yielding under pure torsion occurs when  $\sigma' = \sqrt{3}\tau_{max} = S_y$ 
  - This implies yield strength under pure shear, Ssy, according to DET is;

$$S_{sy} = \frac{S_y}{\sqrt{3}} \cong 0.577S_y$$

- The result  $S_{sy}=0.577S_y$  agrees very well with experiments.
- The DET is also called
  - The octahedral shear stress theory
  - The Von Mises Hencky theory
- From

$$\sigma' = \sqrt{\frac{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_1 - \sigma_3)^2}{2}} = S_y$$

• The equation of the surface enclosing the safe region in stress space is:

$$(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_1 - \sigma_3)^2 = 2S_y^2$$

- This equation describes an obligue elliptical cylinder with hydrostatic line as its axis.
- Safe combinations of principal stresses are inside this cylinder.



 Instead of principal stresses, one can calculate Von Mises stress in terms of a given stress state.

$$\sigma' = \sqrt{\frac{(\sigma_x - \sigma_y)^2 + (\sigma_y - \sigma_z)^2 + (\sigma_x - \sigma_z)^2 + 6(\tau_{xy}^2 + \tau_{yz}^2 + \tau_{xz}^2)}{2}}$$

• Safety Factor can be calculated as;

$$n = \frac{S_y}{\sigma'}$$

 For biaxial case,  $\sigma_{\rm B}$ the safe region is  $S_v$ an elliptical are as shown. Let  $-S_c = S_t = S_y$  $-S_v$  $S_v$  $\sigma_A$  $45^{\circ}$  $-S_v$  $\sigma_A^2 - \sigma_A \sigma_B + \sigma_B^2 = S_v^2$ Loading line for pure torsion

#### **Comparison of Failure Criteria**



# **Comparison of Failure Criteria**

- Note that MNST is actually useful for brittle materials.
- MSST and DET are useful for ductile materials (for predicting onset of yielding).
- MSST is more conservative than DET.
- Agreement of DET with experiments is better.

# **Comparison of Failure Criteria**

• A common type of loading is combined bending and torsion.



- In this case principal stresses are given as  $\sigma_{A}, \sigma_{B} = \frac{\sigma_{x}}{2} \pm \sqrt{\left(\frac{\sigma_{x}}{2}\right)^{2} + \tau_{xy}^{2}}$
- Effective stresses are given as
   MSST: DET:

$$\sigma' = \sigma_A - \sigma_B = 2\sqrt{\left(\frac{\sigma_x}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma' = \sqrt{\sigma_A^2 - \sigma_A \sigma_B + \sigma_B^2} = \sqrt{\sigma_x^2 + 3\tau_{xy}^2}$$